JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
J18-1289-A	T80	39	1	Superior Walls TRUSS DESC.	

Structural Truss Systems, Fort Macleod, Brian Slingerland

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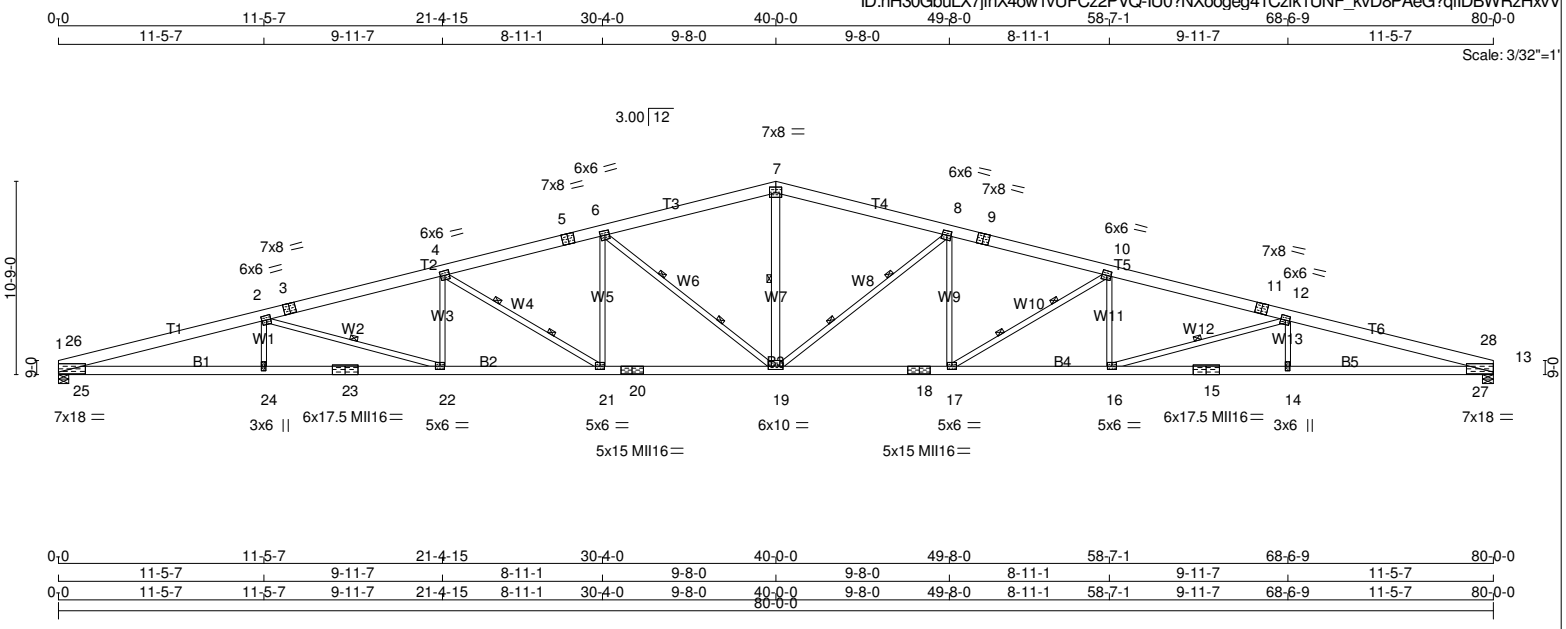

TOTAL WEIGHT = 39 X 463 = 18047 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x8	DRY	1950F 1.7E	SPF
3 - 5	2x8	DRY	1950F 1.7E	SPF
5 - 7	2x8	DRY	1950F 1.7E	SPF
7 - 9	2x8	DRY	1950F 1.7E	SPF
9 - 11	2x8	DRY	1950F 1.7E	SPF
11 - 13	2x8	DRY	1950F 1.7E	SPF
1 - 23	2x6	DRY	2100F 1.8E	SPF
23 - 20	2x6	DRY	2100F 1.8E	SPF
20 - 18	2x6	DRY	2100F 1.8E	SPF
18 - 15	2x6	DRY	2100F 1.8E	SPF
15 - 13	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
19 - 7	2x6	DRY	No.2	SPF
6 - 19	2x4	DRY	2100F 1.8E	SPF
19 - 8	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMB1-I	MT20	7.0	18.0		Edge
2, 4, 6, 8, 10, 12						
2	TMWW-t	MT20	6.0	6.0		
3, 5, 9, 11						
3	TS-t	MT20	7.0	8.0		
7	TTW-p	MT20	7.0	8.0	3.75	4.00
13	TMB1-I	MT20	7.0	18.0		Edge
14	BMW+w	MT20	3.0	6.0		
15	BS-t	MII16	6.0	17.5		
16, 17, 21, 22						
16	BMWW-t	MT20	5.0	6.0		
18	BS-t	MII16	5.0	15.0		
19	BMWWW-t	MT20	6.0	10.0	2.50	5.00
20	BS-t	MII16	5.0	15.0		
23	BS-t	MII16	6.0	17.5		
24	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
1	4257	0	4257	391	-2788	7-4	7-4
13	4257	0	4257	0	-2788	7-4	7-4

PROVIDE ANCHORAGE AT BEARING JOINT 1 FOR 2788 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 2788 LBS FACTORED UPLIFT

PROVIDE FOR 391 LBS FACTORED HORIZONTAL REACTION AT JOINT 1

ALLOW FOR 0.6" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
1	3531	1771 / 0	800 / 0	0 / 0	0 / -2609	960 / 0	0 / 0
13	3531	1771 / 0	800 / 0	0 / 0	0 / -2609	960 / 0	0 / 0
HORIZONTAL REACTIONS							
1	---	0 / 0	0 / 0	0 / 0	280 / -280	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 1, 13

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 3.66 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x4 DRY SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-19, 2-22, 12-16. DBS = 20-0-0 . CBF = 219 LBS.
2 - 2x4 DRY SPF No.2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF 4-21, 10-17. DBS = 16-0-0 . CBF = 211 LBS.
2 - 2x4 DRY SPF No.2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF 6-19, 8-19. DBS = 14-0-0 . CBF = 228 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
1-26	-14192 / 8934	-78.9	-78.9	0.58 (1)	2.90
26- 2	-13876 / 8989	-78.9	-78.9	0.77 (1)	2.94
2- 3	-12497 / 8004	-78.9	-78.9	0.52 (1)	3.28
3- 4	-12497 / 8004	-78.9	-78.9	0.52 (1)	3.28
4- 5	-10691 / 6803	-78.9	-78.9	0.40 (11)	3.65
5- 6	-10691 / 6803	-78.9	-78.9	0.40 (11)	3.65
6- 7	-8637 / 5398	-78.9	-78.9	0.35 (11)	4.02
7- 8	-8637 / 5398	-78.9	-78.9	0.35 (12)	4.02
8- 9	-10691 / 6804	-78.9	-78.9	0.40 (12)	3.65
9-10	-10691 / 6804	-78.9	-78.9	0.40 (12)	3.65
10-11	-12497 / 8005	-78.9	-78.9	0.52 (1)	3.28
11-12	-12497 / 8005	-78.9	-78.9	0.52 (1)	3.28
12-28	-13876 / 8994	-78.9	-78.9	0.77 (1)	2.94
28-13	-14192 / 8939	-78.9	-78.9	0.58 (1)	2.90
1-25	-8846 / 13424	-27.5	-27.5	0.85 (1)	3.77
25-24	-8846 / 13424	-27.5	-27.5	0.85 (1)	3.66
24-23	-8846 / 13424	-27.5	-27.5	0.78 (1)	3.66
23-22	-8846 / 13424	-27.5	-27.5	0.78 (1)	3.66
22-21	-7665 / 12144	-27.5	-27.5	0.66 (1)	3.99
21-20	-6259 / 10371	-27.5	-27.5	0.59 (1)	4.36
20-19	-6259 / 10371	-27.5	-27.5	0.59 (1)	4.36
19-18	-5938 / 10371	-27.5	-27.5	0.59 (1)	4.46
18-17	-5938 / 10371	-27.5	-27.5	0.59 (1)	4.46
17-16	-7344 / 12144	-27.5	-27.5	0.66 (1)	4.07
16-15	-8530 / 13424	-27.5	-27.5	0.78 (1)	3.72

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
24- 2	0 / 435	0.07 (17)
22- 4	-293 / 808	0.13 (5)
21- 6	-746 / 1345	0.61 (13)
19- 7	-2011 / 3535	0.65 (12)
17- 8	-746 / 1345	0.61 (14)
16-10	-294 / 808	0.13 (6)
14-12	0 / 434	0.07 (17)
2-22	-1538 / 1256	0.86 (2)
4-21	-2108 / 1660	0.53 (2)
6-19	-2610 / 2120	0.57 (2)
19- 8	-2610 / 2121	0.57 (3)
17-10	-2108 / 1659	0.53 (3)
16-12	-1538 / 1263	0.86 (3)
25-26	-68 / 992	0.00 (1)
27-28	-68 / 992	0.00 (1)

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	22.2	PSF
	DL =	5.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.1	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 4 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(80 % OF 25.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 22.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.67")
CALCULATED VERT. DEFL.(LL) = L/ 723 (1.33")
ALLOWABLE DEFL.(TL)= L/180 (5.33")
CALCULATED VERT. DEFL.(TL) = L/ 485 (1.98")

CSI: TC=0.77/1.00 (2-26:1) , BC=0.85/1.00 (1-25:1) , WB=0.86/1.00 (12-16:3) , SSI=0.70/1.00 (1-26:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (19) (INPUT = 0.90)
JSI METAL= 0.99 (1) (INPUT = 1.00)

CONTINUED ON PAGE 2

LOADING

TOTAL LOAD CASES: (18)

CHORDS					WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. MEMB. UNBRAC LENGTH FR-TO	FACTORED FORCE (LBS)	MAX CSI (LC)
15-14	-8530 / 13424	-27.5	-27.5	0.78 (1)	3.72		
14-27	-8530 / 13424	-27.5	-27.5	0.85 (1)	3.72		
27-13	-8530 / 13424	-27.5	-27.5	0.85 (1)	3.83		

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING
AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {13.2} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
J18-1289-A	T80GE	2	1	Superior Walls TRUSS DESC.	

Structural Truss Systems, Fort Macleod, Brian Slingerland

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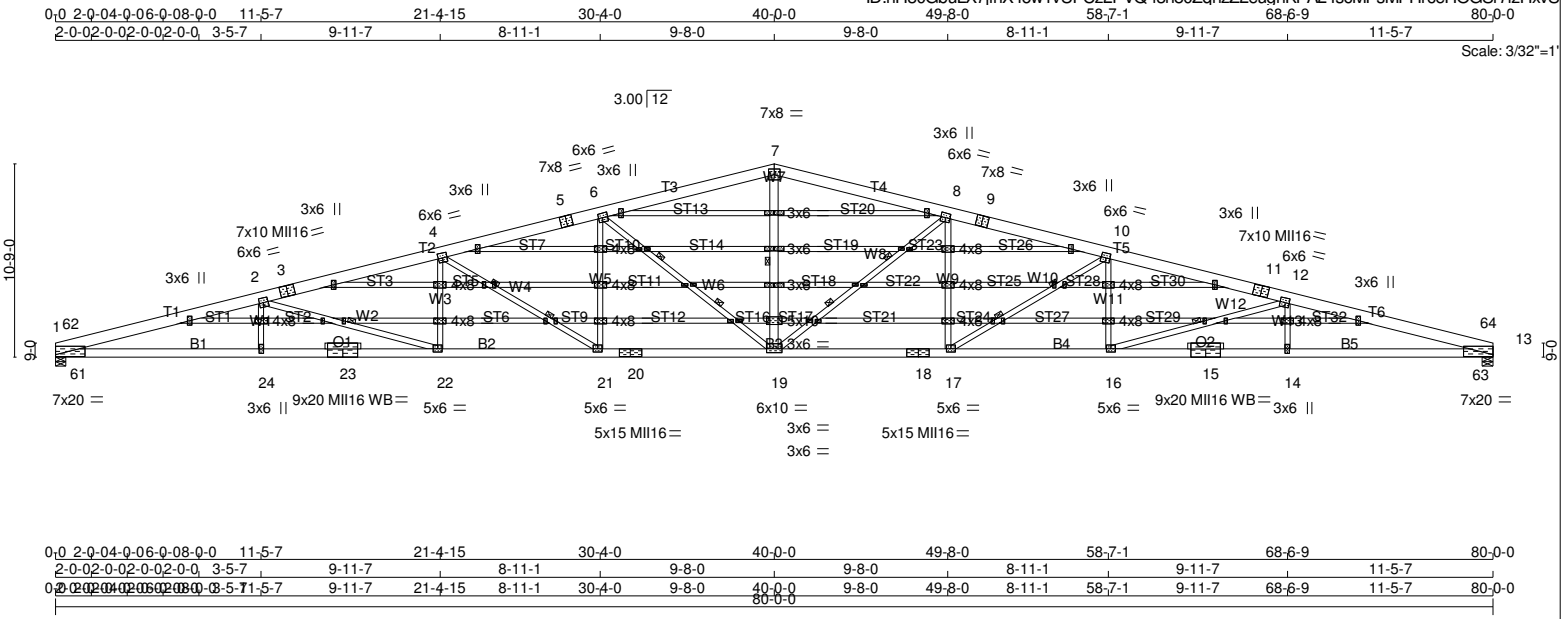

TOTAL WEIGHT = 2 X 652 = 1305 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
1 - 3	2x8	DRY	1950F 1.7E	SPF
3 - 5	2x8	DRY	1950F 1.7E	SPF
5 - 7	2x8	DRY	1950F 1.7E	SPF
7 - 9	2x8	DRY	1950F 1.7E	SPF
9 - 11	2x8	DRY	1950F 1.7E	SPF
11 - 13	2x8	DRY	1950F 1.7E	SPF
1 - 23	2x6	DRY	2100F 1.8E	SPF
23 - 20	2x6	DRY	2100F 1.8E	SPF
20 - 18	2x6	DRY	2100F 1.8E	SPF
18 - 15	2x6	DRY	2100F 1.8E	SPF
15 - 13	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
19 - 7	2x6	DRY	No.2	SPF
6 - 19	2x4	DRY	2100F 1.8E	SPF
19 - 8	2x4	DRY	2100F 1.8E	SPF
ALL GABLE WEBS				
	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
1	TMB1-l	MT20	7.0	20.0		Edge
2, 4, 6, 8, 10, 12						
2	TMWW-t	MT20	6.0	6.0		
3	TS-t	MII16	7.0	10.0		
5	TS-t	MT20	7.0	8.0		
7	TTW-p	MT20	7.0	8.0	3.75	4.00
9	TS-t	MT20	7.0	8.0		
11	TS-t	MII16	7.0	10.0		
13	TMB1-l	MT20	7.0	20.0		Edge
14	BMW+w	MT20	3.0	6.0		
15	BS-t	MII16	9.0	20.0		
16, 17, 21, 22						
16	BMWW-t	MT20	5.0	6.0		
18	BS-t	MII16	5.0	15.0		
19	BMWWW-t	MT20	6.0	10.0	2.75	5.00
20	BS-t	MII16	5.0	15.0		
23	BS-t	MII16	9.0	20.0		
24	BMW+w	MT20	3.0	6.0		
25, 25, 28, 28, 30, 30, 32, 32, 34, 34, 36, 36, 39, 39, 40, 40, 43, 43, 47, 47, 50, 50, 51, 51, 57, 57						
25						
25, 28, 30, 34, 36, 39, 40, 43, 47, 50, 51, 57						
25	WMWW+t	MT20	4.0	8.0		
26, 38, 41, 49, 52, 56, 59, 60						
26	NP+p	MT20	3.0	6.0		
27, 27, 37, 37						
27	NP+p	MT20	2.0	4.0	2.00	0.75
29, 29, 35, 35, 42, 42, 48, 48						
29	NP+p	MT20	2.0	4.0		
31, 31, 33, 33, 44, 44, 46, 46, 53, 53, 55, 55						
31	NP+w	MT20	2.0	4.0		
32	WMWW+t	MT20	5.0	10.0		
45, 45, 54, 54, 58, 58						
45	NP+w	MT20	3.0	6.0	2.50	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
TRUSS ERECTION AND INSTALLATION MUST BE SUPERVISED BY A PROFESSIONAL ENGINEER OR OTHER QUALIFIED PERSON.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
1	4657	0	4657	391	-2788	7-4	7-4
13	4657	0	4657	0	-2788	7-4	7-4

PROVIDE ANCHORAGE AT BEARING JOINT 1 FOR 2788 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT 13 FOR 2788 LBS FACTORED UPLIFT

PROVIDE FOR 391 LBS FACTORED HORIZONTAL REACTION AT JOINT 1

ALLOW FOR 0.6" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
1	3531	1771 / 0	800 / 0	0 / 0	0 / -2609	960 / 0	0 / 0
13	3531	1771 / 0	800 / 0	0 / 0	0 / -2609	960 / 0	0 / 0
HORIZONTAL REACTIONS							
1	---	0 / 0	0 / 0	0 / 0	280 / -280	0 / 0	0 /0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) 1, 13

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 3.66 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 - 2x4 DRY SPF No.2 LATERAL BRACE(S) AT 1/ 2 LENGTH OF 7-19, 2-22, 12-16. DBS = 20-0-0 . CBF = 219 LBS.
2 - 2x4 DRY SPF No.2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF 4-21, 10-17. DBS = 16-0-0 . CBF = 231 LBS.
2 - 2x4 DRY SPF No.2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF 6-19, 8-19. DBS = 12-0-0 . CBF = 213 LBS.

DBS = DIAGONAL BRACE SPACING (MAX). CBF = CUMULATIVE BRACING FORCE (PER BRACE). FASTEN LATERAL BRACE(S) USING (0.122"X3") SPIRAL NAILS : 1 NAIL FOR 2x3 BRACE(S), 2 FOR 1x4, 2x4, 2x5, 3 FOR 2x6, 4 FOR 2x8, 5 FOR 2x10, AND 6 FOR 2x12.

LOADING

TOTAL LOAD CASES: (18)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO						
1-62	-15551 / 8934	-78.9 -78.9	0.65 (1)	2.70		24- 2	0 / 435	0.07 (17)
62- 2	-15179 / 8989	-78.9 -78.9	0.87 (1)	2.75		22- 4	-293 / 919	0.15 (5)
2- 3	-13671 / 8004	-78.9 -78.9	0.60 (1)	3.11		21- 6	-746 / 1540	0.61 (13)
3- 4	-13671 / 8004	-78.9 -78.9	0.60 (1)	3.11		19- 7	-2011 / 3927	0.65 (12)
4- 5	-11694 / 6803	-78.9 -78.9	0.40 (11)	3.49		17- 8	-746 / 1540	0.61 (14)
5- 6	-11694 / 6803	-78.9 -78.9	0.40 (11)	3.49		16-10	-294 / 919	0.15 (6)
6- 7	-9453 / 5398	-78.9 -78.9	0.35 (11)	3.85		14-12	0 / 434	0.07 (17)
7- 8	-9453 / 5398	-78.9 -78.9	0.35 (12)	3.85		2-22	-1663 / 1256	0.93 (2)
8- 9	-11694 / 6804	-78.9 -78.9	0.40 (12)	3.49		4-21	-2307 / 1660	0.59 (2)
9-10	-11694 / 6804	-78.9 -78.9	0.40 (12)	3.49		6-19	-2842 / 2120	0.62 (2)
10-11	-13671 / 8005	-78.9 -78.9	0.60 (1)	3.11		19- 8	-2842 / 2121	0.62 (3)
11-12	-13671 / 8005	-78.9 -78.9	0.60 (1)	3.11		17-10	-2307 / 1659	0.59 (3)
12-64	-15179 / 8994	-78.9 -78.9	0.87 (1)	2.75		16-12	-1663 / 1263	0.93 (3)
64-13	-15551 / 8939	-78.9 -78.9	0.65 (1)	2.70		61-62	-68 / 1081	0.00 (1)
						63-64	-68 / 1081	0.00 (1)
1-61	-8846 / 14684	-37.5 -37.5	0.95 (1)	3.77				
61-24	-8846 / 14684	-37.5 -37.5	0.95 (1)	3.66				
24-23	-8846 / 14684	-37.5 -37.5	0.86 (1)	3.66				
23-22	-8846 / 14684	-37.5 -37.5	0.86 (1)	3.66				
22-21	-7665 / 13286	-37.5 -37.5	0.74 (1)	3.99				
21-20	-6259 / 11343	-37.5 -37.5	0.67 (1)	4.36				
20-19	-6259 / 11343	-37.5 -37.5	0.67 (1)	4.36				
19-18	-5938 / 11343	-37.5 -37.5	0.67 (1)	4.46				
18-17	-5938 / 11343	-37.5 -37.5	0.67 (1)	4.46				
17-16	-7344 / 13286	-37.5 -37.5	0.74 (1)	4.07				
16-15	-8530 / 14684	-37.5 -37.5	0.86 (1)	3.72				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	22.2	PSF
	DL =	5.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.1	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR COMMERCIAL OR INDUSTRIAL BUILDING REQUIREMENTS OF PART 4, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 4 OF BCBC 2018
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- SLOPE REDUCTION FACTOR USED

(80 % OF 25.1 P.S.F. G.S.L. PLUS 2.1 P.S.F. RAIN LOAD) TIMES IMPORTANCE FACTOR EQUALS 22.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (2.67")
CALCULATED VERT. DEFL.(LL) = L/ 723 (1.33")
ALLOWABLE DEFL.(TL)= L/180 (5.33")
CALCULATED VERT. DEFL.(TL) = L/ 485 (1.98")

CSI: TC=0.87/1.00 (2-62:1) , BC=0.95/1.00 (14-63:1) , WB=0.93/1.00 (12-16:3) , SSI=0.73/1.00 (1-62:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

SNOW LOAD IMPORTANCE FACTOR = 1.00
WIND LOAD IMPORTANCE FACTOR = 1.00
LIVE LOAD IMPORTANCE FACTOR = 1.00
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873
MII16	438	302	2547	1256	4283	1816

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (18) (INPUT = 0.90)
JSI METAL= 0.93 (15) (INPUT = 1.00)

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WARNING: THE UNUSUALLY LONG SPAN AND/OR CONFIGURATION OF THIS TRUSS REQUIRES THAT EXTREME CARE BE USED IN ITS APPLICATION. USE PROPER TRANSPORTATION, UNLOADING AND ERECTION METHODS. ASSURE THAT ALL REQUIRED WEB LATERAL BRACING IS COMMUNICATED TO THE BUILDING CONTRACTOR. ENSURE THAT OVERALL BUILDING BRACING IS DESIGNED BY A QUALIFIED ENGINEER, ARCHITECT OR BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (18)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)		MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
15-14	-8530 / 14684	-37.5	-37.5	0.86 (1)		3.72			
14-63	-8530 / 14684	-37.5	-37.5	0.95 (1)		3.72			
63-13	-8530 / 14684	-37.5	-37.5	0.95 (1)		3.83			

TRUSS HAS BEEN CHECKED FOR UNBALANCED LOADING AS PER NBCC 4.1.6.2.(8)

WIND LOAD APPLIED IS DERIVED FROM REFERENCE VELOCITY PRESSURE OF {13.2} PSF AT {40-0-0} FT-IN-SX REFERENCE HEIGHT ABOVE GRADE AND USING EXTERNAL PEAK COEFFICIENTS, CpCg, BASED ON THE {MAIN WIND FORCE RESISTING SYSTEM}.INTERNAL WIND PRESSURE IS BASED ON DESIGN {CATEGORY 2}. BUILDING MAY BE LOCATED ON {OPEN TERRAIN}, AND TRUSS IS DESIGNED TO BE LOCATED AT LEAST {0-0} FT-IN-SX AWAY FROM EAVE.